



GROUND INVESTIGATIONS IRELAND
Geotechnical & Environmental

Catherinestown House,
Hazelhatch Road,
Newcastle,
Co. Dublin.
D22 YD52

Tel: 01 601 5175 / 5176
Email: info@gii.ie
Web: www.gii.ie

Ground Investigations Ireland

Proposed Playground Leixlip, County
Kildare

Kildare County Council

Ground Investigation Report

November 2022





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1.0 Preamble

On the instructions of Kildare County Council, a site investigation was carried out by Ground Investigations Ireland Ltd., in October 2022 at the site of the proposed playground in Leixlip, County Kildare.

2.0 Overview

2.1. Background

It is proposed to construct a new playground with associated services, access roads and car parking at the proposed site. The site is currently occupied by a carpark. The proposed construction is envisaged to consist of conventional foundations and pavement make up with some local excavations for services and plant.

2.2. Purpose and Scope

The purpose of the site investigation was to investigate subsurface conditions utilising a variety of investigative methods in accordance with the project specification. The scope of the work undertaken for this project included the following:

- Visit project site to observe existing conditions
- Carry out 2 No. Plate Tests (CBR's)
- Carry out 2 No. Soakaways to determine a soil infiltration value to BRE digest 365
- Report with recommendations

3.0 Subsurface Exploration

3.1. General

During the ground investigation a programme of intrusive investigation specified by the Consulting Engineer was undertaken to determine the sub surface conditions at the proposed site. Regular sampling and in-situ testing was undertaken in the exploratory holes to facilitate the geotechnical descriptions and to enable laboratory testing to be carried out on the soil samples recovered during excavation and drilling.

The procedures used in this site investigation are in accordance with Eurocode 7 Part 2: Ground Investigation and testing (ISEN 1997 – 2:2007) and B.S. 5930:2015.

3.2. Soakaway Testing

The soakaway testing was carried out in selected trial pits at the locations shown in the exploratory hole location plan in Appendix 1. These pits were carefully excavated and filled with water to assess the infiltration characteristics of the proposed site. The pits were allowed to drain and the drop in water level was recorded over time as required by BRE Digest 365. The pits were logged prior to completing the soakaway test and were backfilled with arising's upon completion. The soakaway test results are provided in Appendix 2 of this Report.

3.3. Insitu Plate Bearing Test

The plate bearing tests were carried out using a 450mm diameter plate at the locations shown on the site plan in Appendix 1. The plate was loaded in increments using a hydraulic jack and an excavator to provide a reaction and the displacement was monitored in accordance with BS1377 Part 9 using independently mounted digital strain gauges. The constrained modulus and equivalent CBR are calculated in accordance with HD29/75 and are provided on the test reports in Appendix 3 of this Report.

4.0 Ground Conditions

4.1. General

The ground conditions encountered during the investigation are summarised below with reference to insitu and laboratory test results. The full details of the strata encountered during the ground investigation are provided in the exploratory hole logs included in the appendices of this report.

The sequence of strata encountered were variable across the site and generally comprised;

- Made Ground
- Cohesive Deposits

MADE GROUND: Made Ground deposits were encountered beneath the and were present to depths of between 0.00m and 0.30mBGL. These deposits were described generally as *Grey fine to coarse subangular to subrounded Gravel*.

COHESIVE DEPOSITS: Cohesive deposits were encountered beneath the Made Ground and were described typically as *brown slightly sandy gravelly CLAY with occasional cobbles*. These deposits had some occasional cobble content, where noted on the exploratory hole logs.

4.2. Groundwater

Groundwater strikes are noted on the exploratory hole logs where they occurred and where possible drilling was suspended for twenty minutes to allow the subsequent rise in groundwater to be recorded. We would point out that these exploratory holes did not remain open for sufficiently long periods of time to establish the hydrogeological regime and groundwater levels would be expected to vary with the tide, time of year, rainfall, nearby construction and other factors.

5.0 Recommendations & Conclusions

5.1. General

The recommendations given and opinions expressed in this report are based on the findings as detailed in the exploratory hole records. Where an opinion is expressed on the material between exploratory hole locations, this is for guidance only and no liability can be accepted for its accuracy. No responsibility can be accepted for conditions which have not been revealed by the exploratory holes. Limited information has been provided at the ground investigation stage and any designs based on the recommendations or conclusions should be completed in accordance with the current design codes, taking into account the variation and the specific details contained within the exploratory hole logs.

5.2. External Pavements

The proposed pavements are recommended to be designed in accordance with the CBR test results included in the Appendices of this Report. The CBR test results indicate that a capping layer or a sufficient depth of crushed stone fill may be required. Plate bearing tests are recommended at the time of construction to verify the design assumptions for the proposed pavement make up and to verify adequate compaction has been achieved. The use of a geogrid and separation membrane may improve the performance of the proposed pavement and enable a more economical pavement design to be achieved, a specialist supplier is recommended to advise of the required strength, depth and type of geotextile for the proposed design.

5.3. Excavations

Short term temporary excavations in the cohesive deposits will remain stable for a limited time only and will require to be appropriately battered or the sides supported if the excavation is below 1.25mBGL or is required to permit man entry. Excavations in the Made Ground will require to be appropriately battered or the sides supported due to the low strength of these deposits. The groundwater and stability noted on the trial pit logs should be consulted when determining the most appropriate construction methods for excavations.

5.4. Soakaway Design

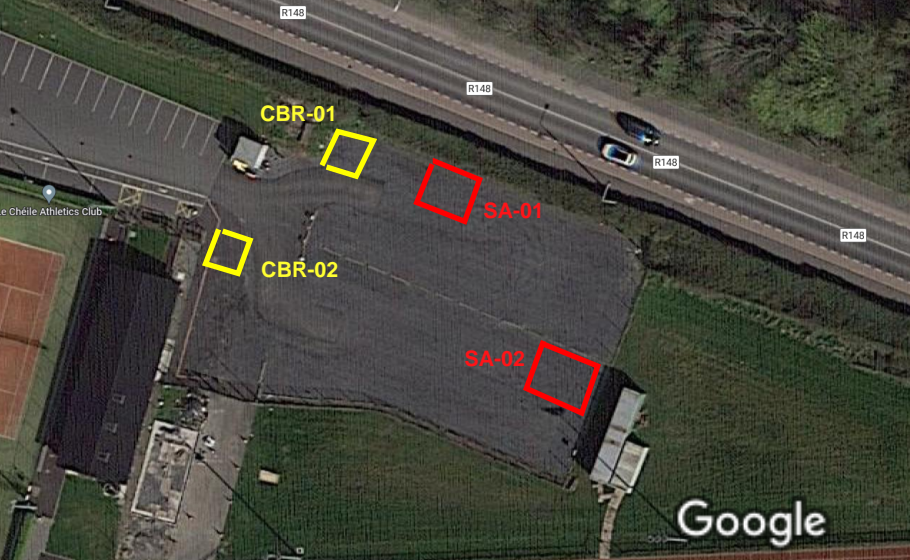
At the locations of SA-01 and SA-02 the water level dropped too slowly to allow calculation of 'f' the soil infiltration rate. These locations are therefore not recommended as suitable for soakaway design and construction.

The recommendations provided in this report should be verified in the design of the proposed buildings, using the full details of the loading conditions and taking into consideration the allowable tolerable settlements/movements that the building can accommodate. The founding strata should be inspected and verified by a suitably qualified engineer prior to construction of the building foundations.

APPENDIX 1 - Site Location Plan



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R148

R148

R148

R148

CBR-01

SA-01

CBR-02

SA-02

Google

le Chéile Athletics Club

APPENDIX 2 – Soakaway Test Records





Machine : JCB Method : Trial Pit		Dimensions (1.90m x 0.50m x 2.00m) (L x W x D)		Ground Level (mOD)		Client Kildare Co. Council		Job Number 12237-09-22	
		Location		Dates 28/10/2022		Project Contractor GII		Sheet 1/1	

Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water
					(0.30)	Grey fine to coarse subangular to subrounded Gravel FILL		
					0.30	Firm brown slightly sandy gravelly cobbly CLAY		
					(0.50)			
					0.80	Stiff brown slightly sandy slightly gravelly CLAY		
					(1.20)			
					2.00	Complete at 2.00m		

Plan .	Remarks Trial pit complete at 2.00m Trial pit stable Groundwater ingress once trial pit was opened Trial pit backfilled upon completion		
	Scale (approx) 1:25	Logged By MK	Figure No. 12237-09-22.SA01

Leixlip Playground Soakaways– Trial Pit Photographs

SA01





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SA01

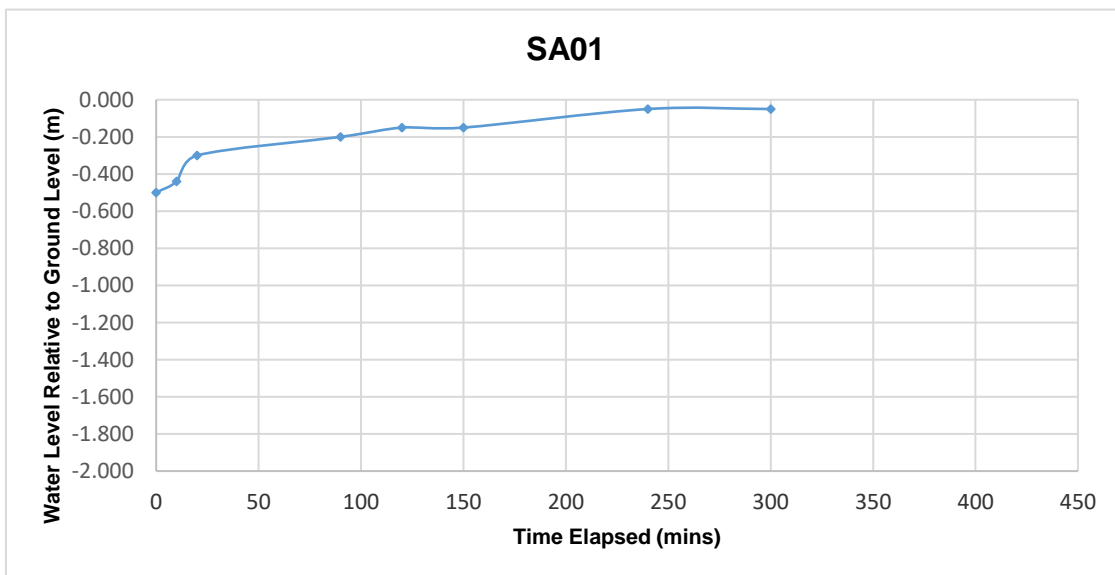
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 1.90m x 0.50m 2.0m (L x W x D)

Date	Time	Water level (m bgl)
28/10/2022	0	-0.500
28/10/2022	10	-0.440
28/10/2022	20	-0.300
28/10/2022	90	-0.200
28/10/2022	120	-0.150
28/10/2022	150	-0.150
28/10/2022	240	-0.050
28/10/2022	300	-0.050

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
0.50	2.000	1.500	0.875	1.625



Leixlip Playground Soakaways– Trial Pit Photographs

SA02





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SA02

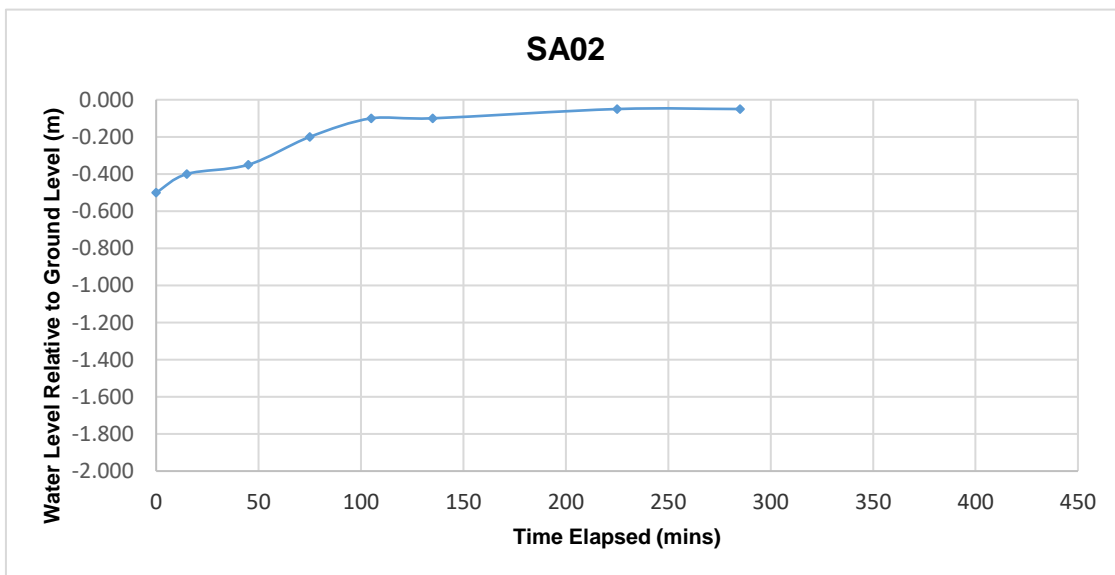
Soakaway Test to BRE Digest 365

Trial Pit Dimensions: 1.80m x 0.45m 2.0m (L x W x D)

Date	Time	Water level (m bgl)
28/10/2022	0	-0.500
28/10/2022	15	-0.400
28/10/2022	45	-0.350
28/10/2022	75	-0.200
28/10/2022	105	-0.100
28/10/2022	135	-0.100
28/10/2022	225	-0.050
28/10/2022	285	-0.050

***Soakaway failed - Pit backfilled**

Start depth	Depth of Pit	Diff	75% full	25%full
0.50	2.000	1.500	0.875	1.625



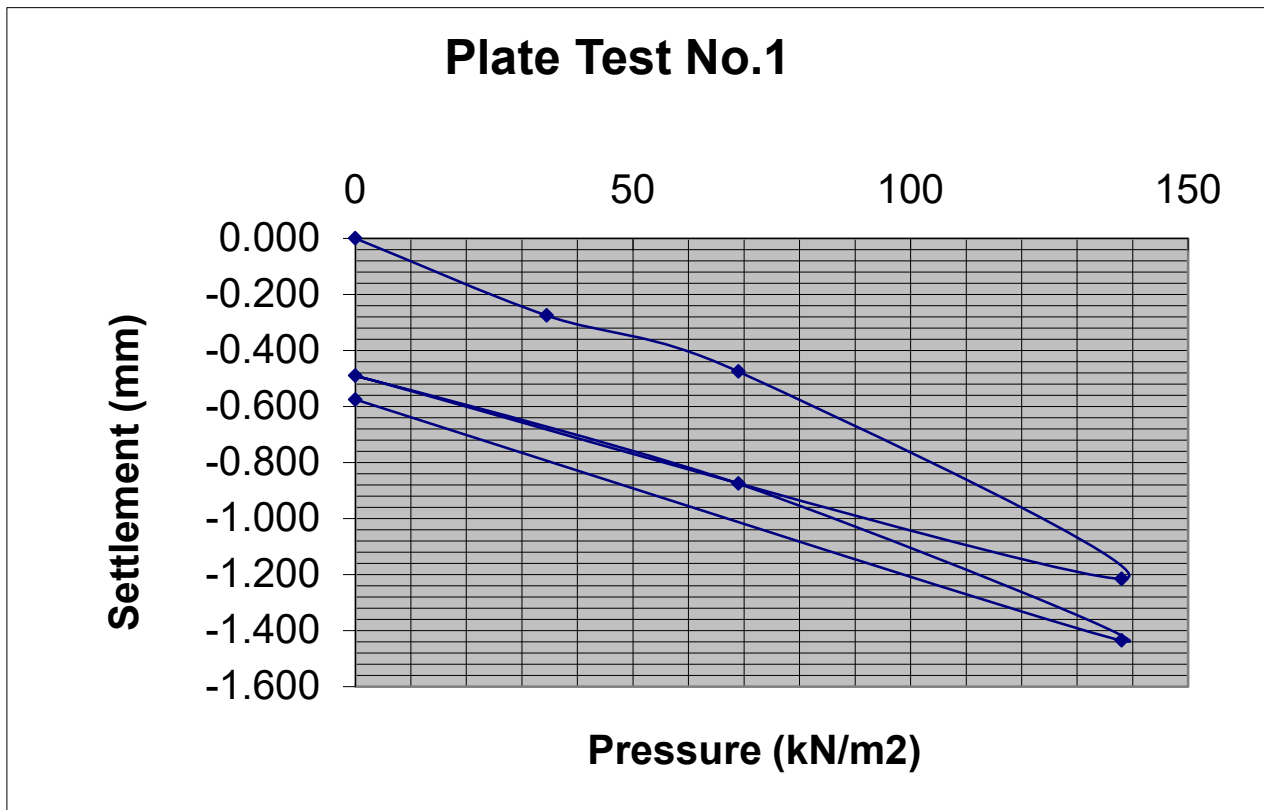
APPENDIX 3 – Plate Test Records



Applied Load	Gauge settlement
0	0.000
34.5	-0.275
69	-0.475
138	-1.215
0	-0.49
69	-0.875
138	-1.435
0	-0.575



LOCATION	Leixlip Playground	MATERIAL	Brown slightly sandy gravelly cobbly
CONTRACT NO.	12237-09-22		CLAY
DATE	28/10/2022	DEPTH	0.50m
CLIENT	Kildare Co. Council	NOTES	
PLATE DIAMETER	305mm	SAMPLES	
TEST NO.	Test 1		



Modulus of subgrade reaction, K (Initial) = **66.02 MN/m²/m**

Modulus of subgrade reaction, K (Reload) = **81.46 MN/m²/m**

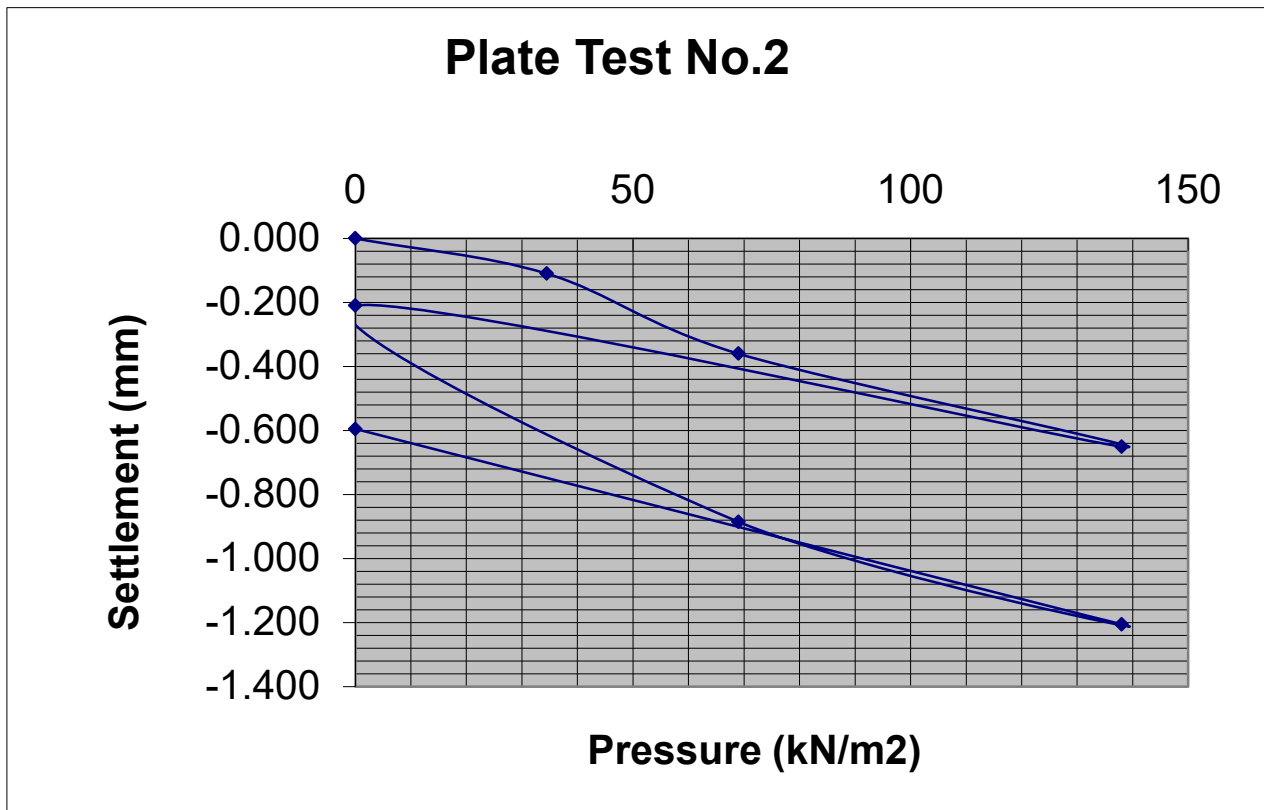
Equivalent CBR(initial)in accordance with HD25/94 volume7 section2 = **13.74 %**

Equivalent CBR(reload)in accordance with HD25/94 volume7 section2 = **19.77 %**

Applied Load	Gauge settlement
0	0.000
34.5	-0.11
69	-0.36
138	-0.65
0	-0.21
69	-0.885
138	-1.205
0	-0.595



LOCATION	Leixlip Playground	MATERIAL	Brown slightly sandy gravelly cobbly CLA'
CONTRACT NO.	12237-09-22		
DATE	28/10/2022		
CLIENT	Kildare Co. Council	DEPTH	0.50m
PLATE DIAMETER	305mm	NOTES	
TEST NO.	Test 1	SAMPLES	



Modulus of subgrade reaction, K (Initial) = **87.11 MN/m²/m**

Modulus of subgrade reaction, K (Reload) = **46.46 MN/m²/m**

Equivalent CBR(initial)in accordance with HD25/94 volume7 section2 = **22.21 %**

Equivalent CBR(reload)in accordance with HD25/94 volume7 section2 = **7.47 %**